

DEVELOPMENT OF FRAMEWORK FOR SEISMIC RESPONSE ANALYSIS OF STORAGE TANK BASED ON FAULT – STRUCTURE SYSTEM

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Abstract

In this study, the framework for the seismic response analysis of oil-storage tank along fault-structure system concept has been presented; where the long period ground motion simulation and sloshing simulation based on 3D finite element method (FEM) is integrated. Three dimensional finite element model is constructed using multi-resolution structured and unstructured mesh for the ground motion prediction. For the sloshing analysis Lagrangian Finite Element Method is used. Also the effect of surface topography on the sloshing response is checked and the effect is found to be significant.

Keywords: earthquake response, long period ground motion, FEM, storage tank.

1. Introduction

The seismic performance of an oil-storage tank has been of particular interest since its failure may cause a great disaster. In 2003 Tokachi-Oki earthquake the oil tanks in and around Tomakomai area were severely damaged. Similar instances of damage could be seen in 1964 Niigata earthquake, 1983 Nihonkai-Chubu earthquake, 1993 Southwest-Off Hokkaido earthquake, 1994 Alaska earthquake (USA), 1999 Koceaeli earthquake (Turkey), 1999 Chi-Chi earthquake (Taiwan) and so on [Zama et al (2008)].

To reduce the earthquake disaster we need to know the seismic behavior of such structures accurately for probable large earthquakes. For the accurate estimation of sloshing phenomenon we need the ground motion caused by the site-specific earthquake.

But recent researches show that the effect of surface topography has significant effect on earthquake ground motion distribution [Ma et al (2006), Lee et al (2009), Pokharel et al (2011)]. Thus, we use 3D FEM for the ground motion simulation, which can handle the complicated surface and interface geometry.

In general, the structural seismic response is a consequence of the following processes: the rupture of a fault, propagation of earthquake waves through the crust, amplification of the waves near the ground surface, and soil-structure interaction. A complete model for all of these processes is a fault-structure system, and must include a fault, crust structure, surface ground layers, and target structures. One of the candidates for analyzing such fault-structure system is three-dimensional (3D) numerical simulation. With the advancement of computational technology and development of 3D soil and crust structure, we can create fairly complex and more realistic models and do 3D numerical simulations with higher accuracy and resolution. There are several researches on

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Earthquake ground motion simulation usually neglects the effect of surface topography.

seismic structural response simulations based on 3D numerical simulation along fault-structure system concept (e.g. Ichimura et al (2012)).

The content of this article include the following: first in methodology section, outline of long period ground motion simulation and sloshing simulation is presented. Then, in numerical experiment section, long period ground motion simulation and sloshing response of Akita oil storage tank is presented.

2. Methodology

For the ground motion simulation a 3D FEM is used. This method solves the problem by generating a hybrid mesh to handle the complicated geometry using structured and unstructured tetrahedral and cubic elements to handle the complicated surface and interface geometry.

In this study, 3D FEM based on elastodynamics is used. The working equation is derived by discretizing in space by Galerkin's method and in time by Newmark- β ($\beta = 1/4$) method with a Rayleigh damping term added

$$\left(\mathbf{K} + \frac{2}{\Delta t} \mathbf{C} + \frac{4}{\Delta t^2} \mathbf{M} \right) \mathbf{u}^{n+1} = \left(\frac{2}{\Delta t} \mathbf{C} + \frac{4}{\Delta t^2} \mathbf{M} \right) \mathbf{u}^n + \left(\mathbf{C} + \frac{4}{\Delta t} \mathbf{M} \right) \mathbf{v}^n + \mathbf{M} \mathbf{a}^n + \mathbf{f}^n$$

Therein, \mathbf{K} , \mathbf{C} , \mathbf{M} , \mathbf{u} , \mathbf{v} , \mathbf{a} , \mathbf{f} , Δt and n are the stiffness matrix, damping matrix, mass matrix, displacement vector, velocity vector, acceleration vector, force vector, time increment and time step respectively. Rayleigh damping model is used to calculate the damping matrix.

$$\mathbf{C} = \alpha \mathbf{K} + \beta \mathbf{M}$$

Therein, α and β are the scalar constant and calculated as in, Bielak (2006).

Since the size of target domain is larger and the resolution of discretization is finer, the computation cost of the modeling and simulation will be huge. To reduce this computation cost, we use highly-tuned FEM [Ichimura et al

(2009)]. This FEM solves the problem 1) by generating a hybrid mesh to handle the complicated geometry using multi-resolution cubic elements and unstructured tetrahedral elements for modeling and 2) by applying element-by-element method and conjugate gradient method to the solver.

For the sloshing analysis, we use 3D FEM based on the fluid-structure coupling analysis method, where the Lagrange-type and the Euler-type finite element formulations are utilized for structure and fluid modeling, respectively [Nagashima and Tsukuda, 2010].

3. Numerical Experiment

In this section, as an application example, we conduct the response analysis of Akita oil storage tank (located at the northern part of Honsu island, Japan) in the past-observed earthquake that occurred in the Akita region on 2008/06/14 with magnitude of M7.2 epicenter at (39.0298°N, 140.8807°E). This application example includes the earthquake ground motion simulation and sloshing simulation.

Firstly, the simulation of long period ground motion distribution is conducted. The analysis domain is 120km×120km×100km in EW, NS and UD direction respectively. The analysis domain is presented in **Fig 1** where target domain is depicted by dotted line, earthquake epicenter by star, observation stations by triangles and location of oil-tank by circle with cross. The mechanism solution is estimated by F-NET. The crust structure is obtained from J-SHIS which consists of 10 layers. The element size is set such that the shear wavelengths corresponding to target frequency are resolved with 10 elements per one wavelength. The target frequency is set at 0.25Hz and band-pass filter is applied to remove the frequency components except [0.0-0.25] Hz. **Fig 2** shows the close up view of the generated mesh. The analysis is done using time increment of 0.01sec for the target duration of 81.92sec. Absorbing boundary conditions are applied at the side and bottom of the domain to implement the infinite extent of domain. **Fig 3** shows the comparison of

calculated and observed waveforms at one of the K-NET observation station in EW, NS and UD component. As can be seen in the Fig, amplitude

and phase property are reproduced with comparatively good accuracy.

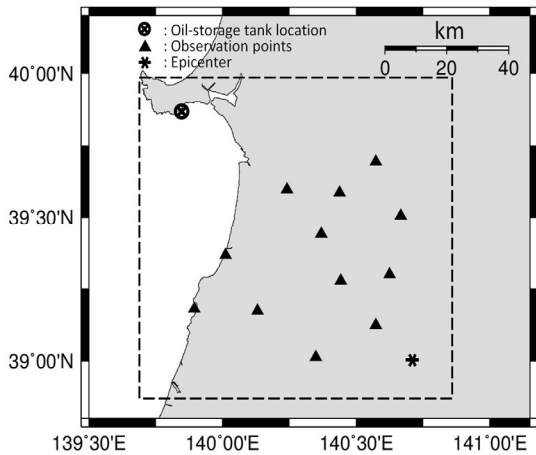


Fig 1 Analysis domain. Dashed line, star, black triangle and circle with cross represents the analysis domain, epicenter of earthquake, observation locations and location of oil tank respectively.

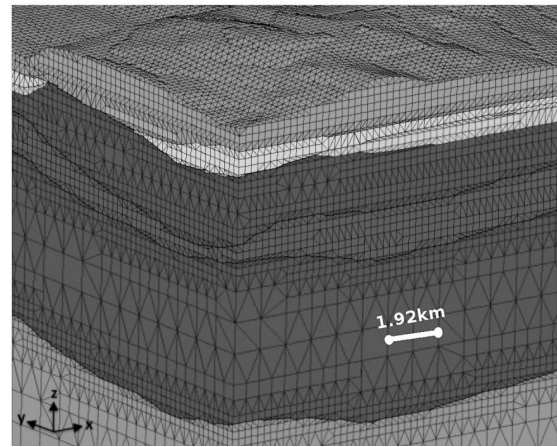


Fig 2 Close up view of generated mesh, which shows the hybrid mesh generated for the analysis to handle the complicated geometry of real topography of the analysis region.

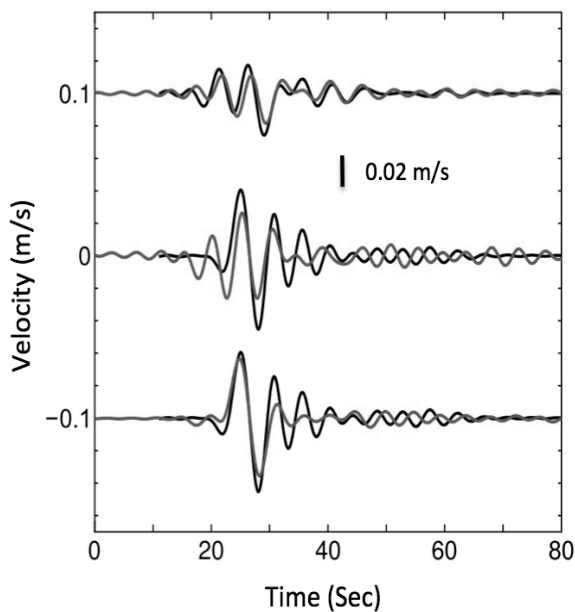


Fig 3 Velocity time series of a K-NET station, at EW (lower), NS (middle), and UD (upper) components in m/s, where black line represents the observed data and red line represents the synthetic data. The horizontal axis is the time in seconds.

The seismic response analysis of oil storage tank is conducted by using the ground motion

simulation result for Akita earthquake obtained in previous section. Here, the time history of acceleration is used for the external excitation. We construct a fictitious 3D FEM model for storage tank as shown in Fig 4, which is equivalent to existing tank. The storage tank model resembling the actual size of 100,000-kiloliter class stockpiled floating roof type oil storage tank with single deck is used in the analysis. The tank has an internal radius of 40m and height of 25m. Liquid level of the tank is 20m from the bottom of the tank. The tank material is steel and thickness at bottom is 9mm and wall thickness is 14mm. The maximum sloshing height is calculated caused by the above-mentioned earthquake. The maximum sloshing height obtained from this analysis is 18.85cm, which is very close to the observed height. Here, the sloshing height is calculated for two cases viz, with considering the surface topography and flat surface approximation. The maximum sloshing height considering the surface topography is found to be more than 20% greater than that of flat surface approximations.

4. Conclusions

We present a framework for estimation of sloshing response in an earthquake which integrate ground motion simulation and sloshing simulation based on 3D FEM so that we can

obtain the result with higher accuracy. As an application example, earthquake ground motion simulation of the observed earthquake is conducted and the computed wave profiles are compared with the observed wave profiles.

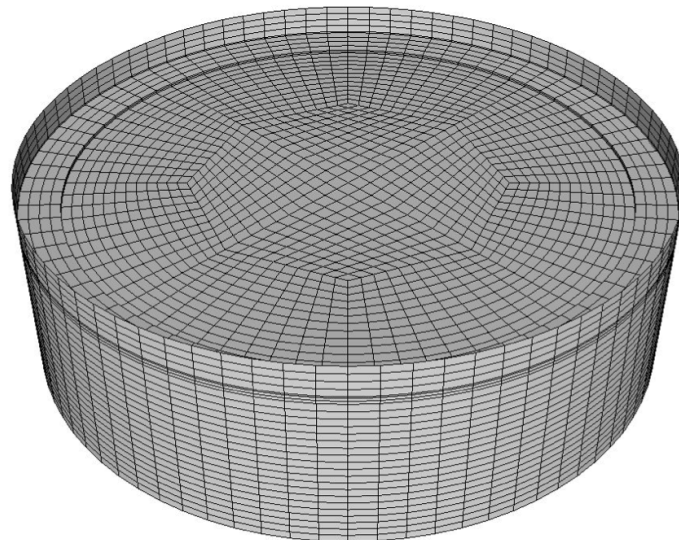
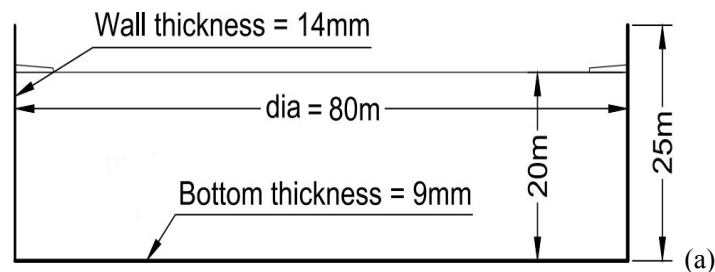


Fig 4 Dimension (a) and FEM mesh (b) of single deck floating roof type oil-storage tank used in this study, which resembles the actual size of 100,000-kiloliter class stockpiled storage tank.

After checking the closer agreement between the observed and the computed wave profiles, we conduct the seismic response of Akita oil-storage tank in that observe earthquake. We also check the effect of surface topography on the sloshing response of Akita oil tank. Although in this particular case of a small earthquake, the difference in sloshing height with/without considering the surface topography is not large, the effect is expected to be significant in case of

large earthquakes. In the next step, we are planning to conduct seismic response analysis of oil tank in large earthquake using our framework.

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